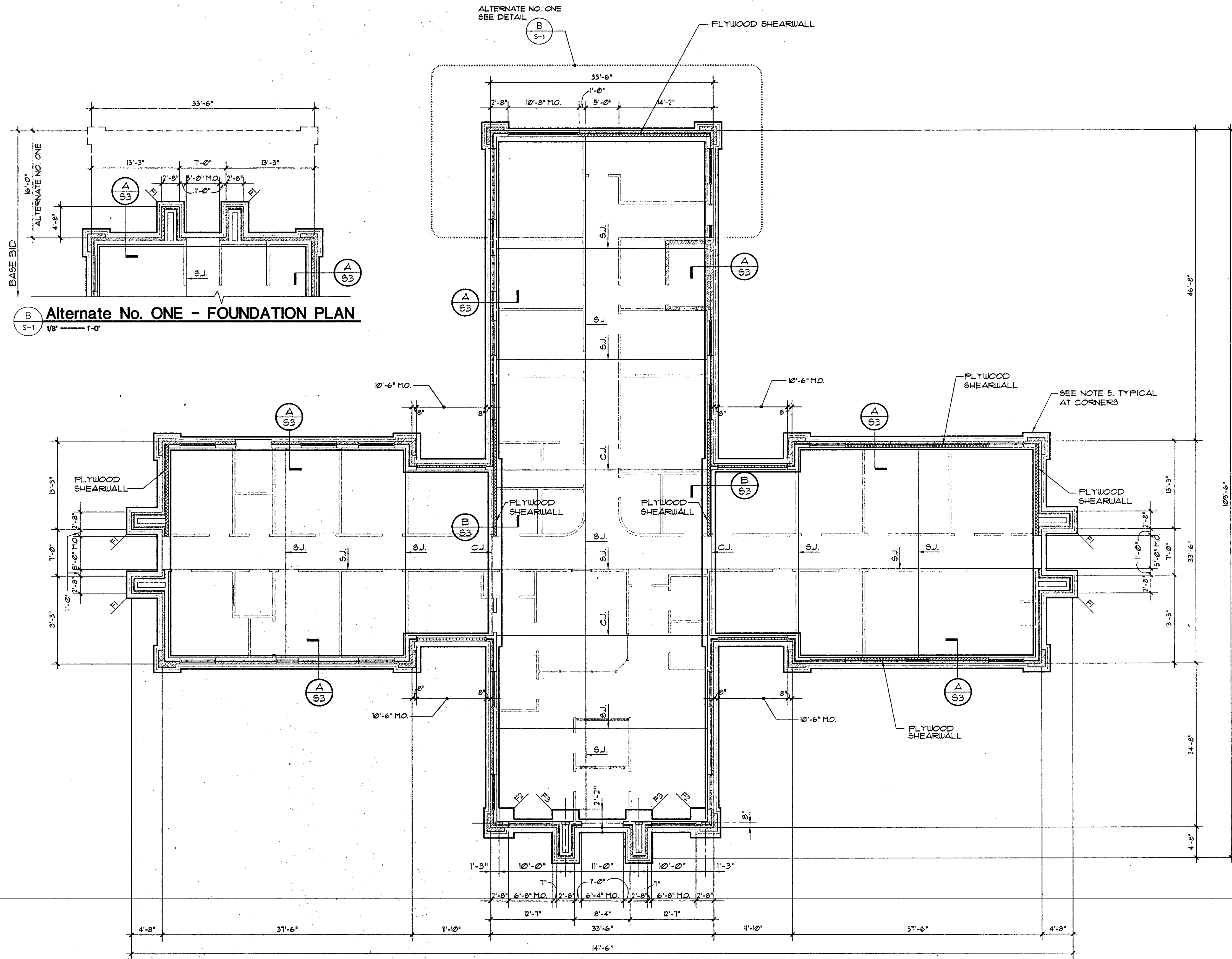


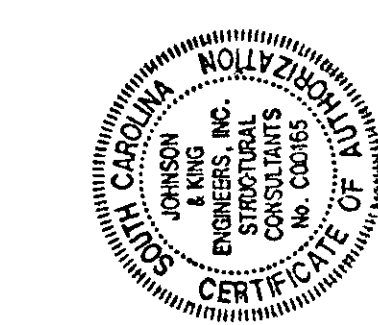
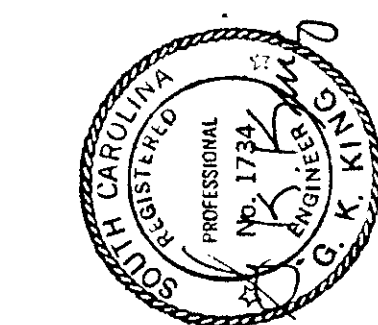


A FOUNDATION PLAN
S-1 1/8" = 1'-0"

- NOTES:
1. FINISH FLOOR ELEVATION - SEE CIVIL DRAWINGS.
 2. FLOOR SLAB CONSTRUCTION - 4" SLAB ON GRADE REINFORCED WITH WUF 6x6 - W14 X W14 OVER VAPOR BARRIER AND 4" SAND BASE.
 3. TOPS OF FOOTINGS - 1'-4" BELOW FINISH FLOOR UNLESS NOTED OTHERWISE.
 4. SEE DWG. 53 FOR SAW JOINT (S.J.), CONSTRUCTION JOINT (C.J.), AND STEP FOOTING DETAILS AND FOOTING SCHEDULE.
 5. WIDEN EXTERIOR FOOTINGS BY 8" TO THE EXTERIOR SIDE AT ALL CORNERS FOR 4'-0" IN EACH DIRECTION.

Lancaster Satellite **DMH**
CATAWBA M.H.C.
Lancaster, S.C.

STATE PROJECT NO. J12-9592-PC
Commission: C93-1510
Date: OCT. 23, 1995



Design Collaborative

ARCHITECTS PLANNERS / COLUMBIA SOUTH CAROLINA

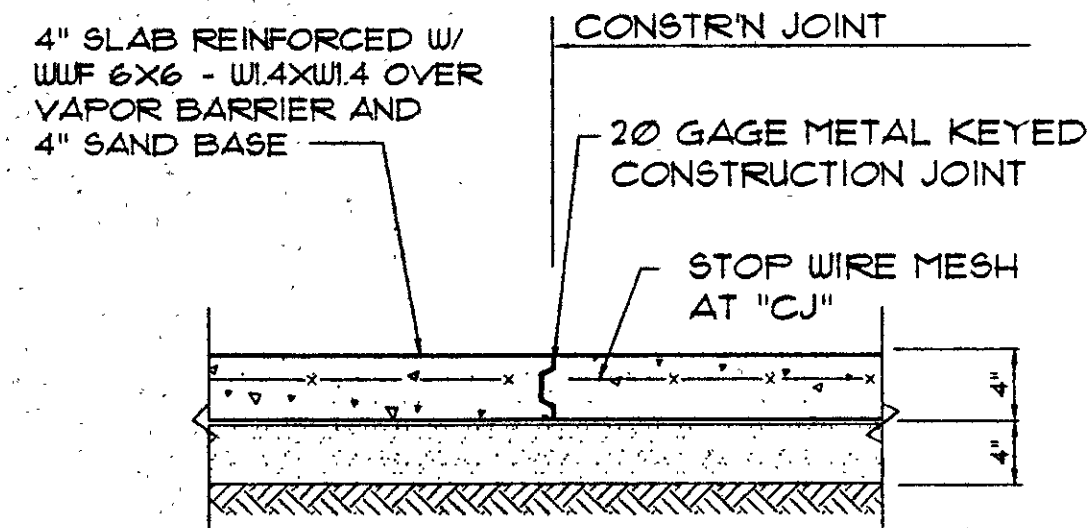
Description	Revision



- PLAN NOTES:
1. ROOF CONSTRUCTION IS $\frac{3}{8}$ " PLYWOOD SHEATHING OVER WOOD TRUSSES SPACED @ 24" OC MAXIMUM.
 2. TRUSS BEARING ELEVATION IS +12'-0" ABOVE FINISH FLOOR UNLESS NOTED OTHERWISE.
 3. BARREL VAULT FRONT ENTRY HAS TWO LAYERS OF $\frac{3}{8}$ " PLYWOOD SUPPORTED BY 2X8 PURLINS @ 24" OC MAX.
 4. SEE E/54 FOR SHEARWALL SCHEDULE AND DETAILS.
 5. ALL GIRDER TRUSSES AND PARALLAM BEAMS SHALL BEAR ON (3) 2X6 STUDS AT EACH END.
 6. CONNECT JACK TRUSSES TO GIRDER TRUSS WITH SIMPSON LUS24 HANGERS.
 7. CONNECT HIP JACK TO GIRDER TRUSS WITH SIMPSON LTHJ HANGER.
 8. GIRDER TRUSSES MUST BE 2-PLY MINIMUM WITH 2X6 MINIMUM BOTTOM CHORD.
 9. 2X6 AT FRONT OF BUILDING IS CONTINUOUS OVER ALL 4 CI COLUMNS.
 10. SEE DWG. 54 FOR COLUMN SCHEDULE.
 11. STICK FRAME ENTRY ROOF IS 2X6 RAFTERS @ 16" OC MAX. ANCHOR TO STUD WALLS WITH SIMPSON HI ANCHORS.
 12. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS AND DETAILS OF DRAFTSTOPPING IN ATTIC.

OPENING WIDTH	LINTEL	END BEARING
4'-0" OR LESS	L3½ X3½ X¼	8"
4'-1" TO 6'-0"	L4X3½ X5/16LLV	8"
6'-1" TO 10'-8"	L6X3½ X5/16LLV	8"

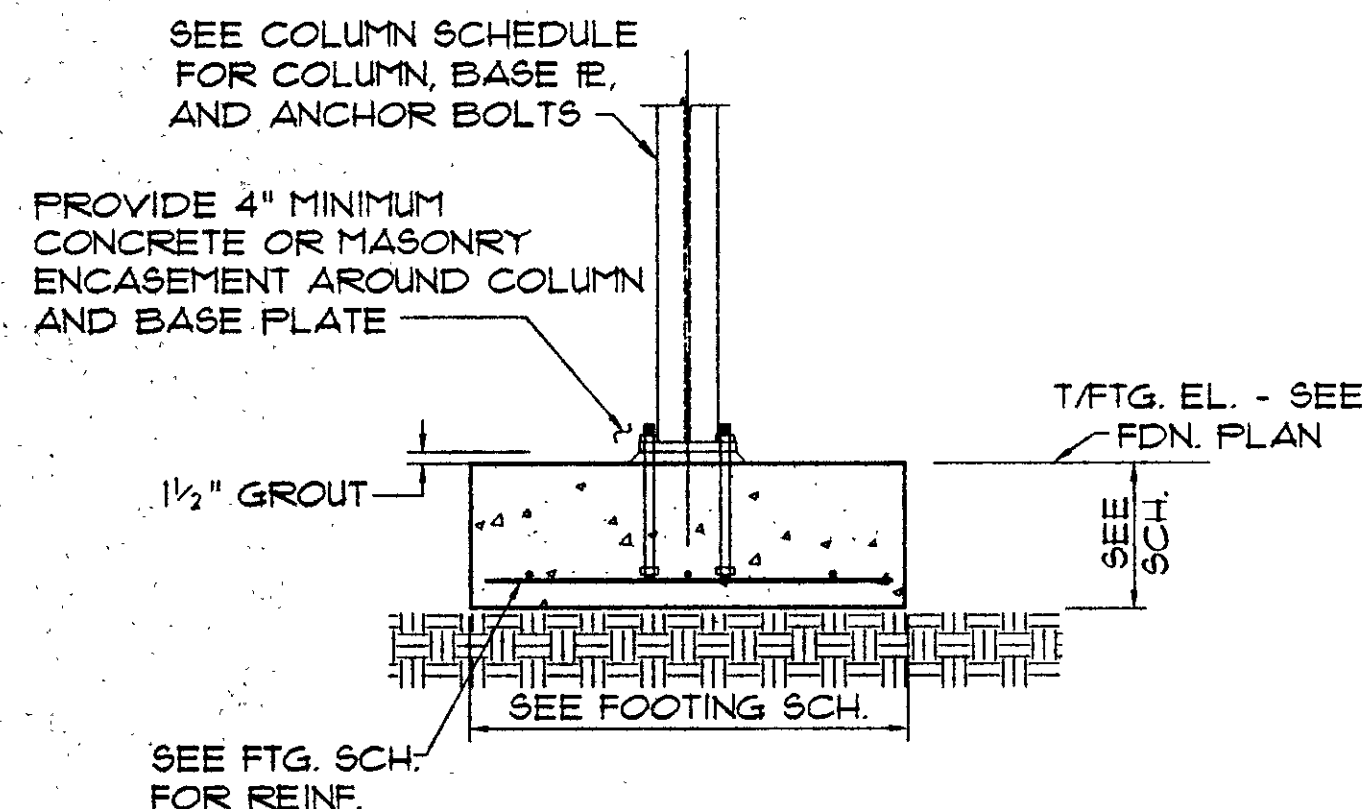
NOTE: USE THESE LINTELS AT OPENINGS IN BRICK WALLS WHERE OTHER LINTELS ARE NOT SHOWN ON PLAN OR DETAILS. SEE ARCH DWGS. FOR SIZE AND LOCATIONS OF OPENINGS.



CONSTR'N JOINT DETAIL

SCALE 3/4" = 1'-0"
NOTED "CJ" ON FOUNDATION PLAN

FOOTING SCHEDULE			
MARK	SIZE	THICKNESS	REIN. EA. WAY
F1	4'-0" X 6'-8"	12"	4 #5 LONG WAY 1 #5 SHORT WAY
F2	4'-6" X 4'-6"	12"	4 #5
F3	4'-0" X 8'-0"	12"	4 #5 LONG WAY 8 #5 SHORT WAY

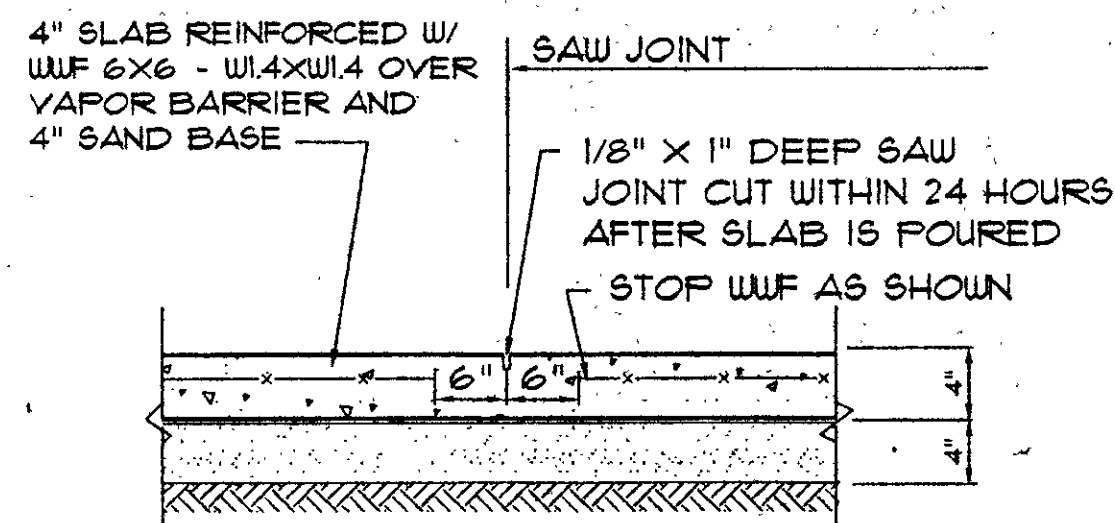


TYPICAL COLUMN BASE AND FOOTING DETAIL

3/4" = 1'-0"

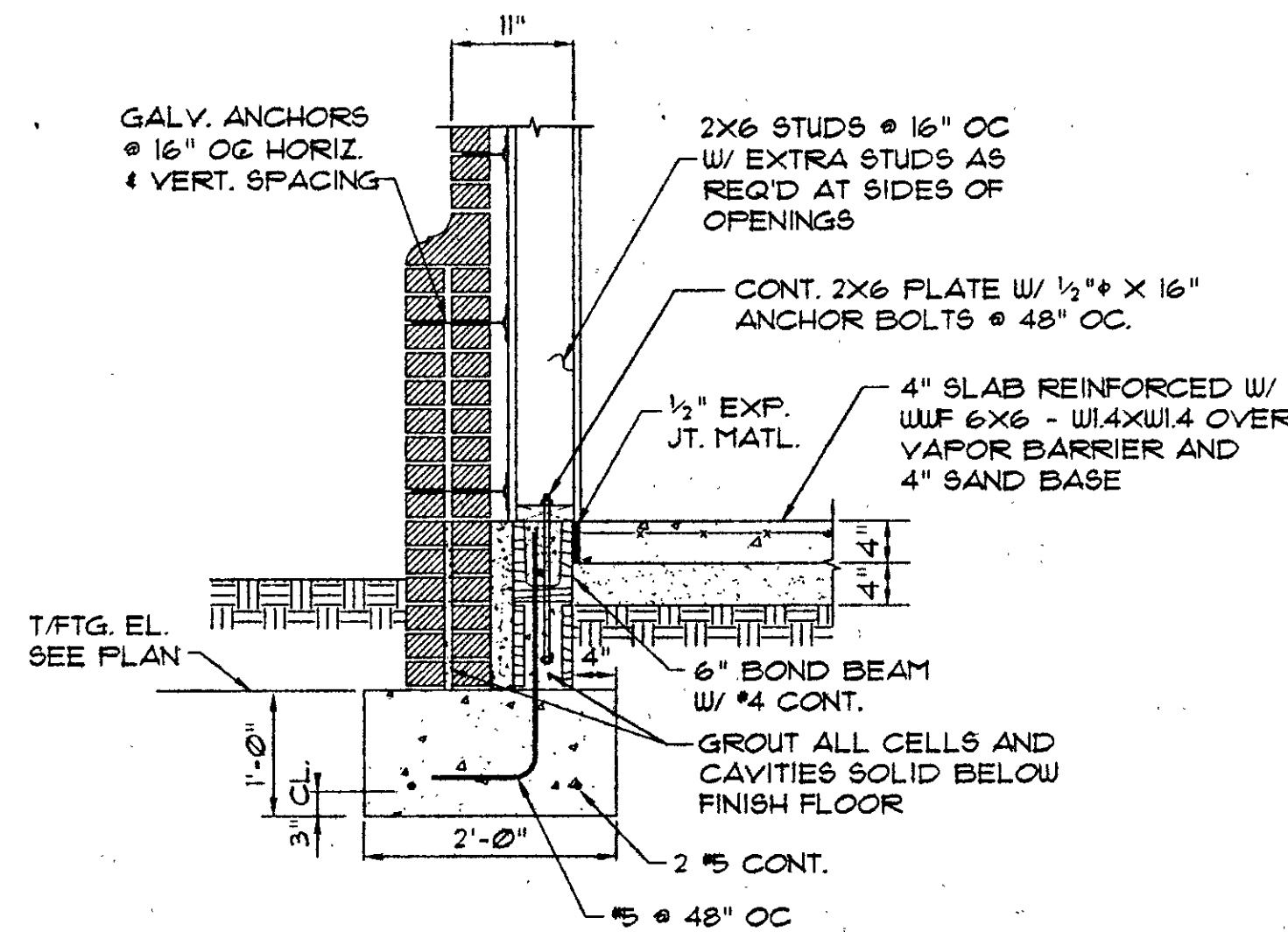
COLUMN SCHEDULE			
	C1		
T/STL OF BEAMS	41'-5"		
	W 14X22		
FIRST FLOOR			
T/FTG. (SEE FDN. PLAN)			
BASE IR	1"X8"X16"		
ANCHOR BOLTS	4 - 3/4"X1'-2"		
DETAIL	D/54		
NOTES			

NOTES: 1. ANCHOR BOLTS ARE ASTM A36 OR ASTM A307.
2. STEEL COLUMNS SHALL BE ENCASED IN 4" MINIMUM OF CONCRETE OR MASONRY BELOW GRADE.



SAW JOINT DETAIL

SCALE 3/4" = 1'-0"
NOTED "SJ" ON FOUNDATION PLAN

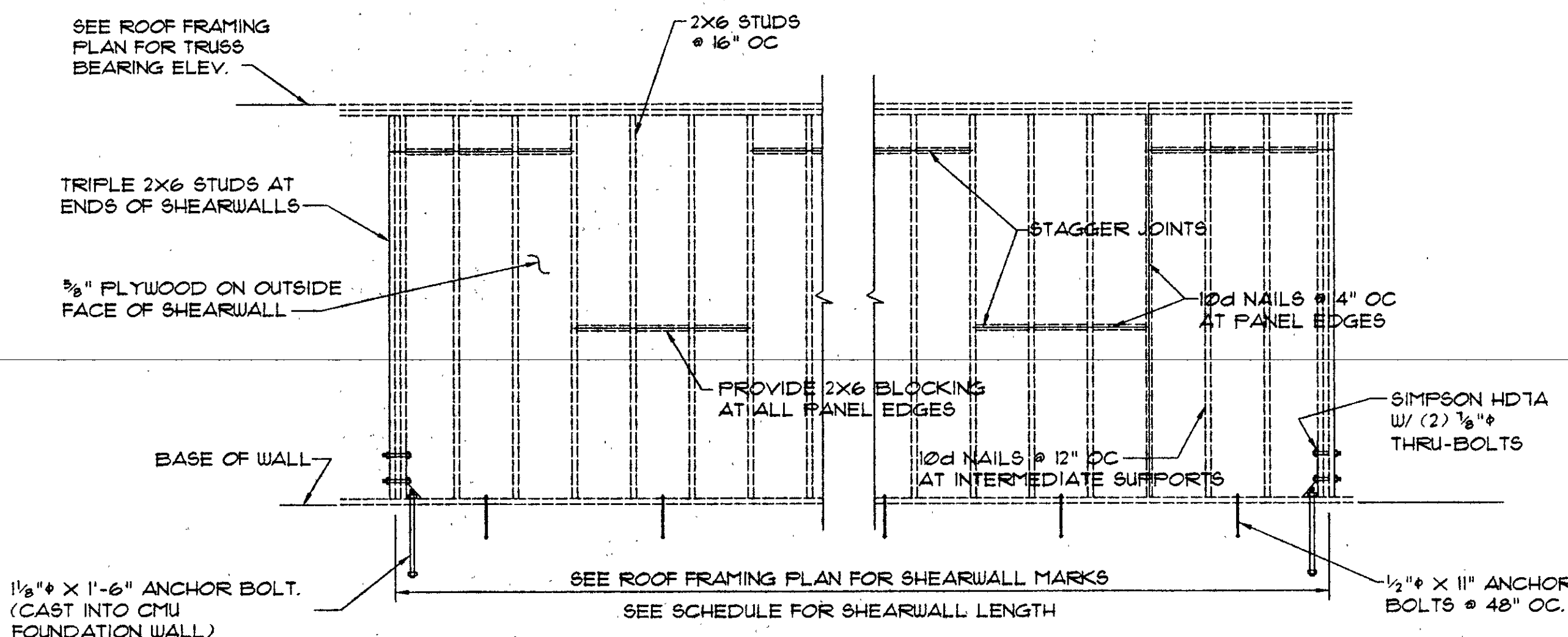


SECTION A-53

SCALE 3/4" = 1'-0"

NOTES:
1. FLYWOOD SHEATHING FOR SHEARWALLS NOT SHOWN. SEE DWG. 52
FOR LOCATIONS OF SHEARWALLS. SEE C/53 FOR SHEARWALL DETAILS
AND SPECIAL ANCHOR BOLTS REQUIRED AT SHEARWALLS.

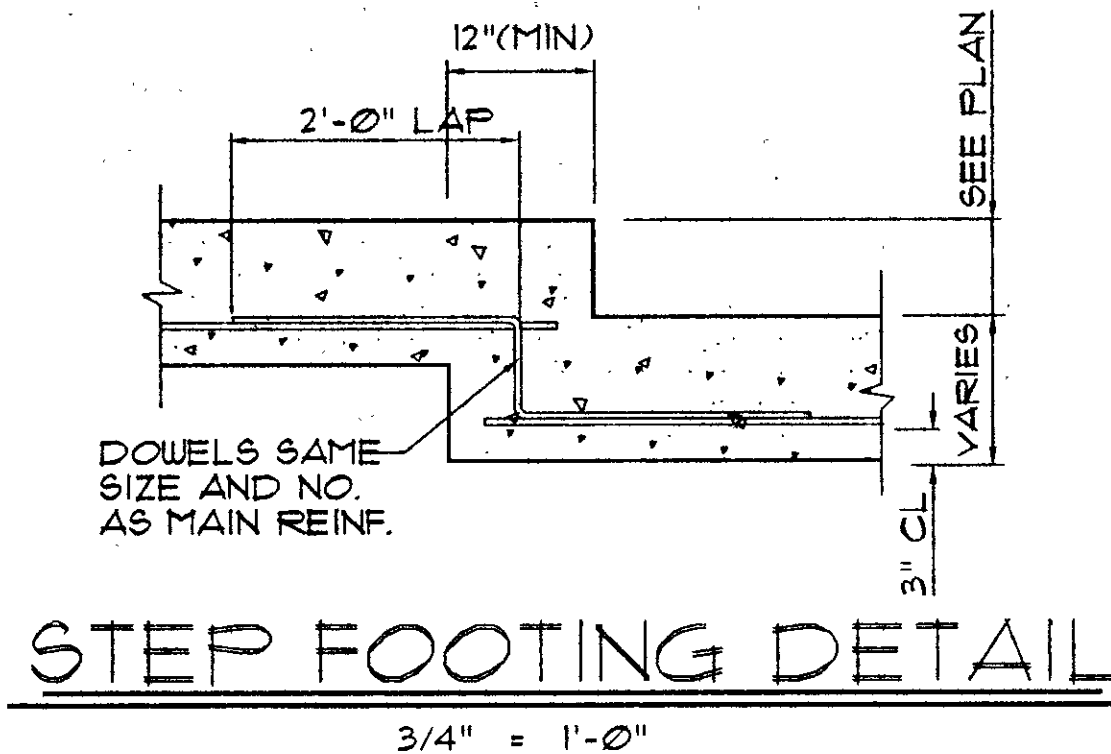
SHEARWALL SCHEDULE				
SHEARWALL MARK	"A"	"B"	"C"	"D"
SHEARWALL LENGTH	17'-6"	19'-8"	13'-9"	21'-6"
NUMBER REQUIRED	2	1	2	2



DETAIL C-53

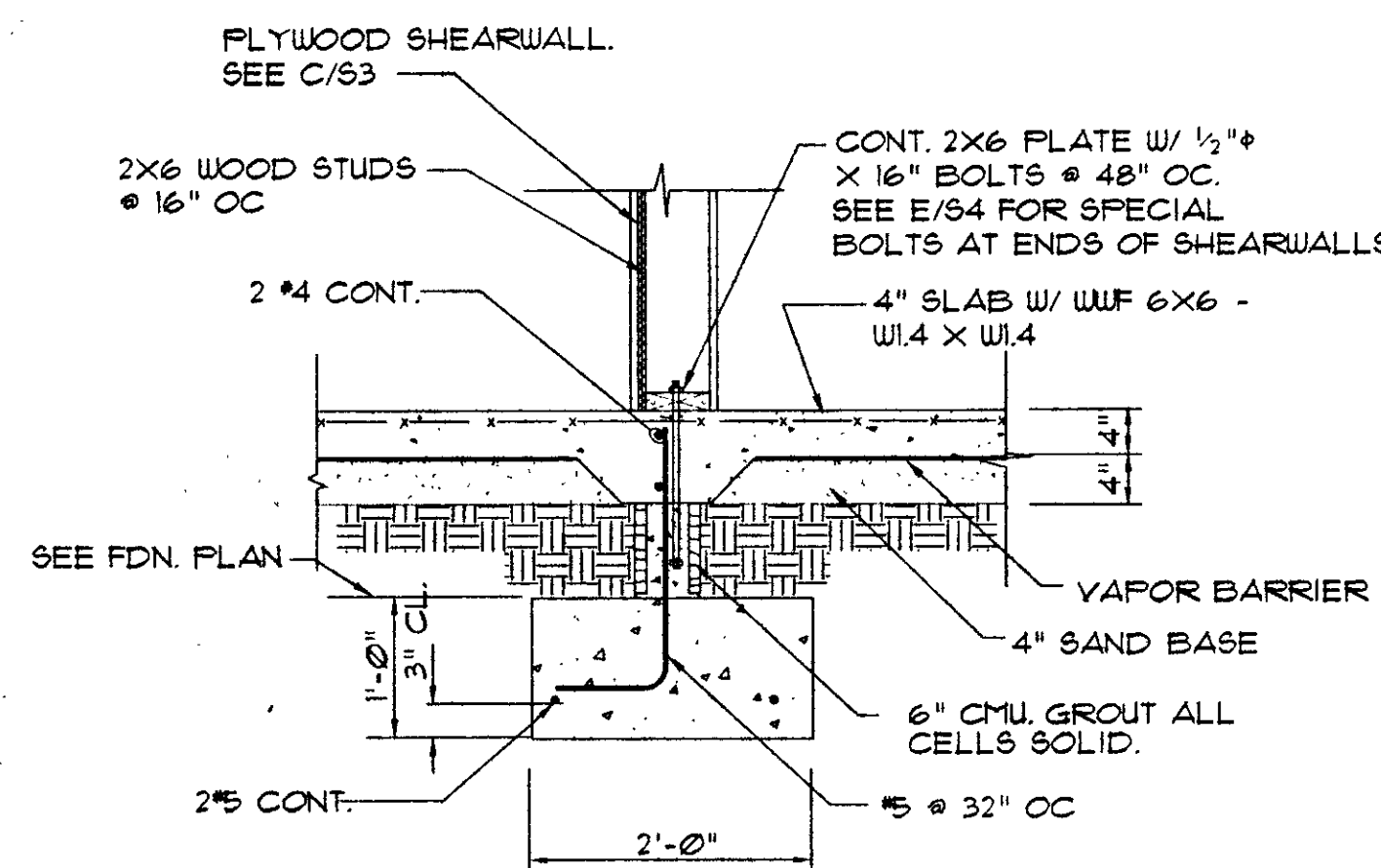
SCALE 3/8" = 1'-0"

NOTES: 1. PROVIDE DOUBLE 2X6 FRAMING AROUND ALL OPENINGS.
NAIL WITH 10d NAILS @ 4" OC AROUND ALL SIDES OF OPENINGS.



STEP FOOTING DETAIL

3/4" = 1'-0"



SECTION B-53

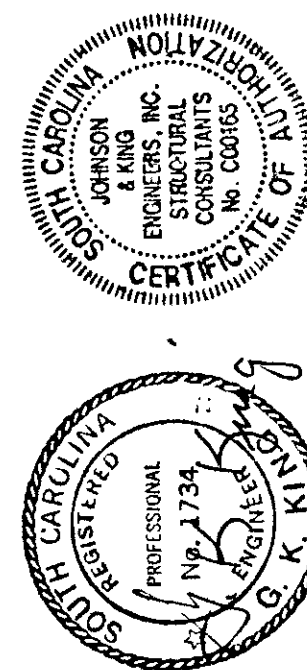
SCALE 3/4" = 1'-0"

- GENERAL NOTES:
- IN CASE OF A DISCREPANCY BETWEEN THE ARCHITECTURAL AND STRUCTURAL DRAWINGS, CONSULT WITH THE ARCHITECT. FOR DIMENSIONS OR DETAILS NOT SHOWN, SEE THE ARCHITECTURAL DRAWINGS.
 - DESIGN CRITERIA:
 - BUILDING CODE - 1994 STANDARD BUILDING CODE.
 - SEISMIC DESIGN DATA:
 - AV. LESS THAN 0.15
 - AS. LESS THAN 0.15
 - SEISMIC HAZARD EXPOSURE GROUP: I
 - SEISMIC PERFORMANCE CATEGORY: C
 - SOIL PROFILE TYPE: S = 2.0 (ASSUMED)
 - BASIC STRUCTURAL SYSTEM: BEARING WALL SYSTEM
 - SEISMIC RESISTING SYSTEM: LIGHT FRAMED WALLS WITH SHEAR PANELS
 - RESPONSE MODIFICATION FACTOR: R = 6.5
 - DEFLECTION AMPLIFICATION FACTOR: Cd = 4
 - ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE PROCEDURE.
 - WIND VELOCITY 80 MPH
 - ROOF LIVE LOAD - 20 PSF
 - FOUNDATION DESIGN IS BASED ON A SUBSURFACE INVESTIGATION BY GEO-SYSTEMS DESIGN & TESTING, INC. DATED FEBRUARY 8, 1995.
 - DESIGN BEARING PRESSURE IS 3000 PSF
 - STEP FOOTINGS AS REQUIRED TO LET UTILITIES PASS OVER FOOTINGS.
 - CONCRETE CLASSES AND 28 DAY STRENGTHS ARE TO BE AS FOLLOWS:
 - ALL CONCRETE - 3000 PSI (150 PCF)
 - CONCRETE REINFORCING STEEL - ASTM A615, GRADE 60.
 - DETAIL IN ACCORDANCE WITH ACI DETAILING MANUAL, LATEST EDITION.
 - LAP ALL BARS WITH CLASS "B" SPLICES UNLESS NOTED OTHERWISE.
 - PROVIDE CORNER BARS OF SAME SIZE AND SPACING AS HORIZONTAL BARS AT ALL WALL INTERSECTIONS. LAP WITH CLASS "B" SPLICES.
 - STRUCTURAL STEEL:
 - PIPE - ASTM A501, OR A53 GRADE B
 - TUBE - ASTM A500, GRADE B
 - OTHER - ASTM A36
 - FABRICATION & ERECTION IN ACCORDANCE WITH AISC SPECIFICATIONS.
 - BOLTED CONNECTIONS - A325N HIGH STRENGTH BOLTS.
 - WELDED CONNECTIONS E70XX ELECTRODES.
 - UNLESS DETAILED OTHERWISE, SHOP CONNECTIONS TO BE WELDED, FIELD CONNECTIONS TO BE BOLTED.
 - WOOD CONSTRUCTION:
 - WOOD TRUSSES - ALL PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN SOUTH CAROLINA FOR THE FOLLOWING LOADS AND ANY OTHER SPECIAL LOADS SHOWN ON DRAWINGS:
 - TOP CHORD DEAD LOAD - 15 PSF.
 - TOP CHORD LIVE LOAD - 20 PSF.
 - BOTTOM CHORD DEAD LOAD - 10 PSF.
 - TOTAL LOAD - 45 PSF.
 - WIND LOADS - PER DESIGN CRITERIA IN SECTION 2
 - TRUSS MANUFACTURER SHALL SUBMIT COMPLETE TRUSS SHOP DRAWINGS SHOWING MATERIAL, PLATE DATA, STRESSES, BRACING AND ERECTION DETAILS FOR APPROVAL BEFORE TRUSSES ARE FABRICATED. BRACING SHOWN ON DRAWINGS IS IN ADDITION TO ANY BRACING REQUIRED BY THE TRUSS DESIGNER.
 - FRAMING LUMBER - SOUTHERN PINE NO. 2, 19% MAXIMUM MOISTURE CONTENT. ALL FRAMING SHALL BE DONE IN ACCORDANCE WITH THE STANDARD BUILDING CODE.
 - WHERE SPECIFIC TRUSS HANGERS ARE NOT SHOWN, SIMPSON HANGERS OF APPROPRIATE TYPE AND CAPACITY SHALL BE PROVIDED.
 - REINFORCED MASONRY:
 - CONCRETE MASONRY UNITS (CMU) SHALL BE NORMAL WEIGHT UNITS IN ACCORDANCE WITH ASTM C90, GRADE N, TYPE I.
 - MORTAR FOR REINFORCED MASONRY SHALL BE TYPE S IN ACCORDANCE WITH ASTM C710.
 - GROUT SHALL BE 2500 PSI FEA GRAVEL CONCRETE IN ACCORDANCE WITH ASTM C416, (8" TO 11" SLUMP).
 - LAP ALL #4 BARS 2'-0", LAP ALL #5 BARS 2'-6" MINIMUM.

- SITE PREPARATION NOTES
- UNDER THE BUILDING AND TO A LINE 10 FEET BEYOND THE BUILDING THE FOLLOWING SITE WORK SHALL BE PERFORMED:
- REMOVE ALL TOPSOIL TO A MINIMUM 6 INCH STRIPPING DEPTH AND STOCKPILE.
 - REMOVE ALL UNSUITABLE SURFACE MATERIALS FROM SITE. THIS INCLUDES SURFACE VEGETATION, ORGANIC-LADEN TOPSOIL, TREE STUMPS AND ROOT MATS, AND ANY UNSTABLE SURFACE MATERIALS.
 - PROOFROLL THE TOP SURFACE OF THE REMAINING MATERIAL WITH A PNEUMATIC TIRED ROLLER (I.E. FULLY LOADED DUMP TRUCK, GRADING PAN, ETC.) IN TANDEN WITH A HEAVY SHEEPSFOOT ROLLER MAKE AT SIX PASSES OVER EACH AREA WITH THE LAST THREE PASSES PERPENDICULAR TO THE FIRST THREE PASSES. ANY AREAS WHICH ARE FOUND TO PUMP OR DEFLECT EXCESSIVELY SHALL BE STABILIZED BY FURTHER IN-PLACE COMPACTION OR SHALL BE UNDERCUT AND BACKFILLED AS IN 5.
 - ONLY LABORATORY-APPROVED FILL MATERIALS SHALL BE USED AS BACKFILL. SEE THE SUBSURFACE INVESTIGATION REPORT FOR THIS PROJECT FOR A DESCRIPTION OF ACCEPTABLE FILL MATERIALS AND LIMITATIONS ON MOISTURE CONTENT AND CONTROL.
 - PLACE NEW LABORATORY-APPROVED FILL MATERIAL IN 6" LAYERS AND COMPACT TO 95% OF ITS STANDARD PROCTOR MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D-698. THE UPPER 12 INCHES OF FINISHED GRADE SHALL BE COMPACTED TO 100% OF ITS STANDARD PROCTOR DENSITY VALUE IN ACCORDANCE WITH ASTM D-698. COMPACTION WILL REQUIRE USE OF A PNEUMATIC TIRE COMPACTOR IN TANDEN WITH A HEAVY SHEEPSFOOT ROLLER. FOOTING BOTTOMS SHALL BE COMPACTED WITH A SMALL MECHANICAL COMPACTOR.
 - NOTE THAT THE SOILS ON THIS SITE MAY REQUIRE DISCING/AERATING OR BLENDING WITH OFF-SITE DRYER SANDS TO DECREASE THEIR MOISTURE CONTENT AND PLASTICITY. BLENDING AND AERATING MAY SLOW DOWN THE EARTHWORK. SEE THE SUBSURFACE INVESTIGATION REPORT FOR MORE INFORMATION.
 - ALL NEW FILL IS TO BE LABORATORY TESTED FOR COMPACTION. PROVIDE ONE COMPACTION TEST FOR EACH 2500 SQUARE FEET OF FILL AREA FOR EACH ONE FOOT VERTICAL OF FILL. LOCATION OF TESTS TO BE AT RANDOM AND AT THE DISCRETION OF THE TESTING LABORATORY. THE TESTING LABORATORY SHALL VERIFY FOOTING BOTTOMS FOR THE DESIGN BEARING PRESSURE.

Design Collaborative

ARCHITECTS PLANNERS / COLUMBIA SOUTH CAROLINA

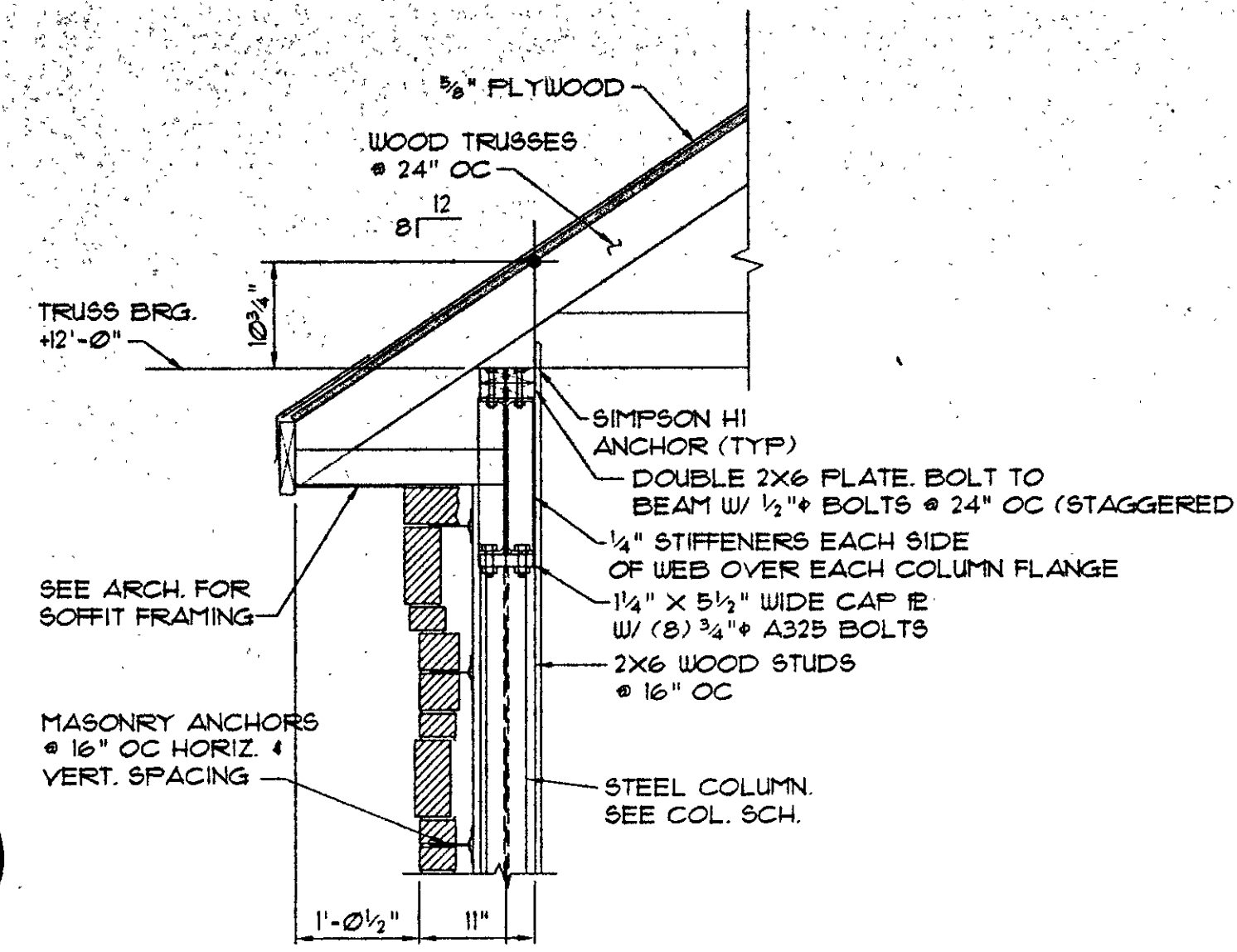
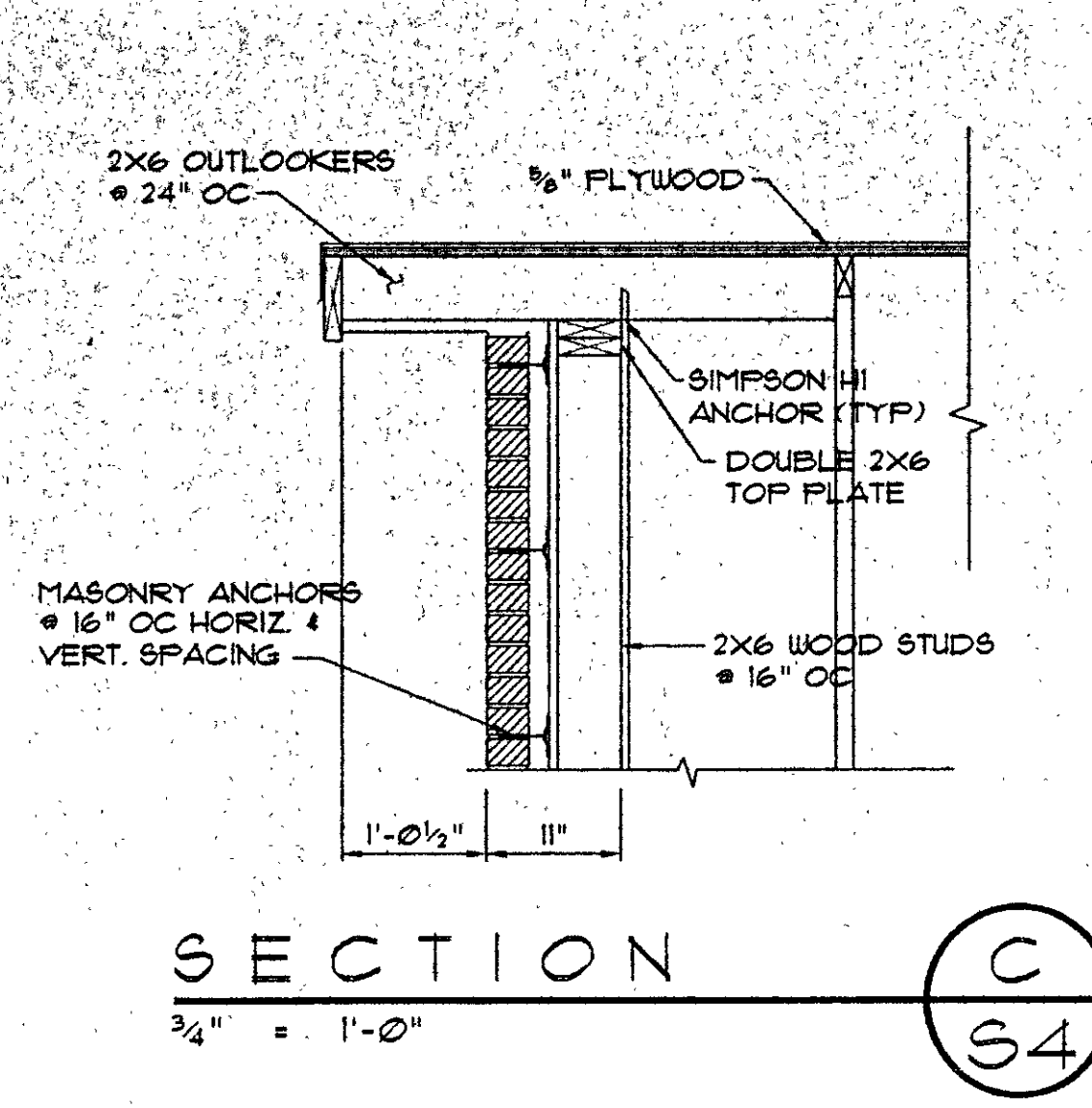
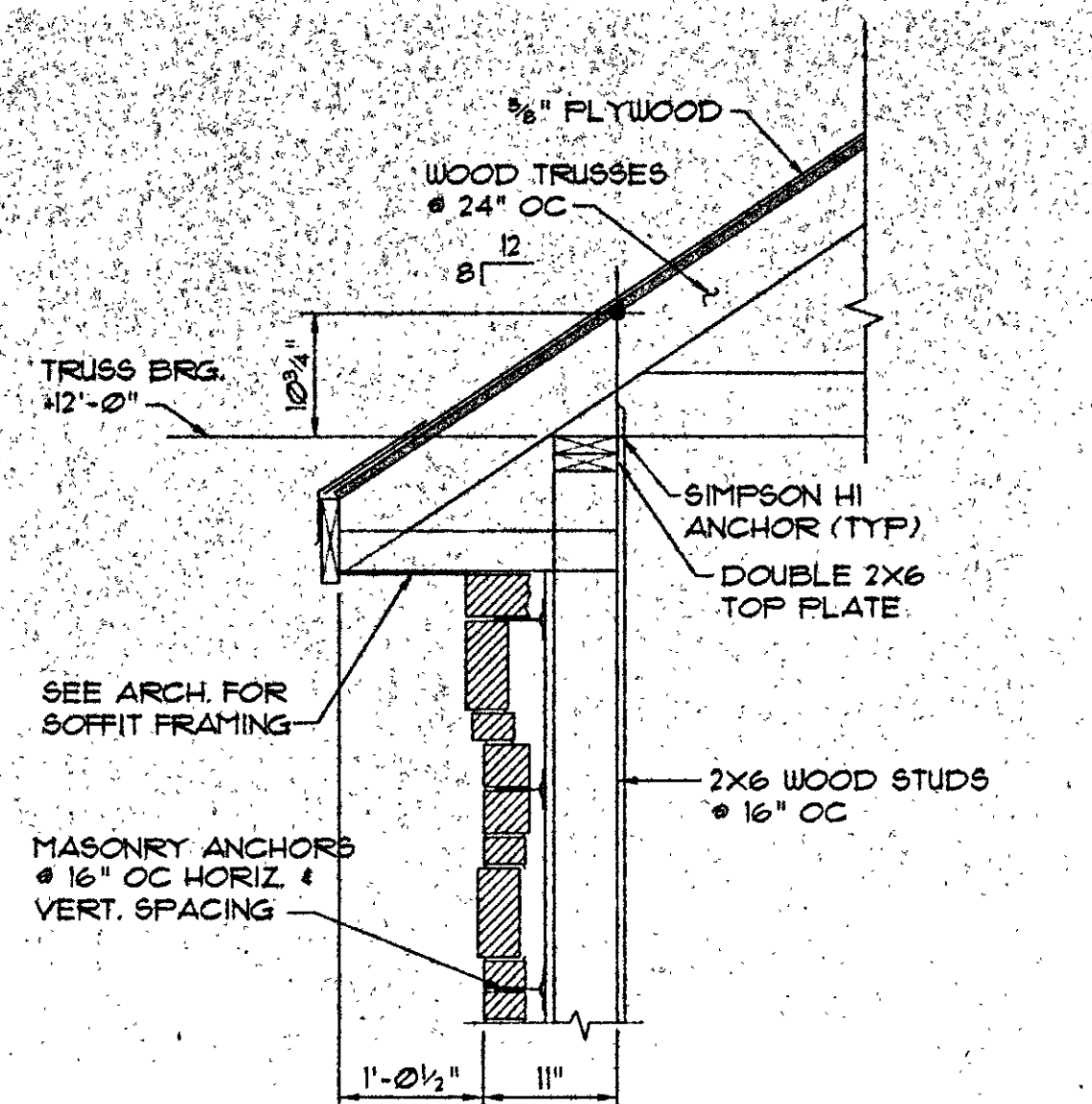
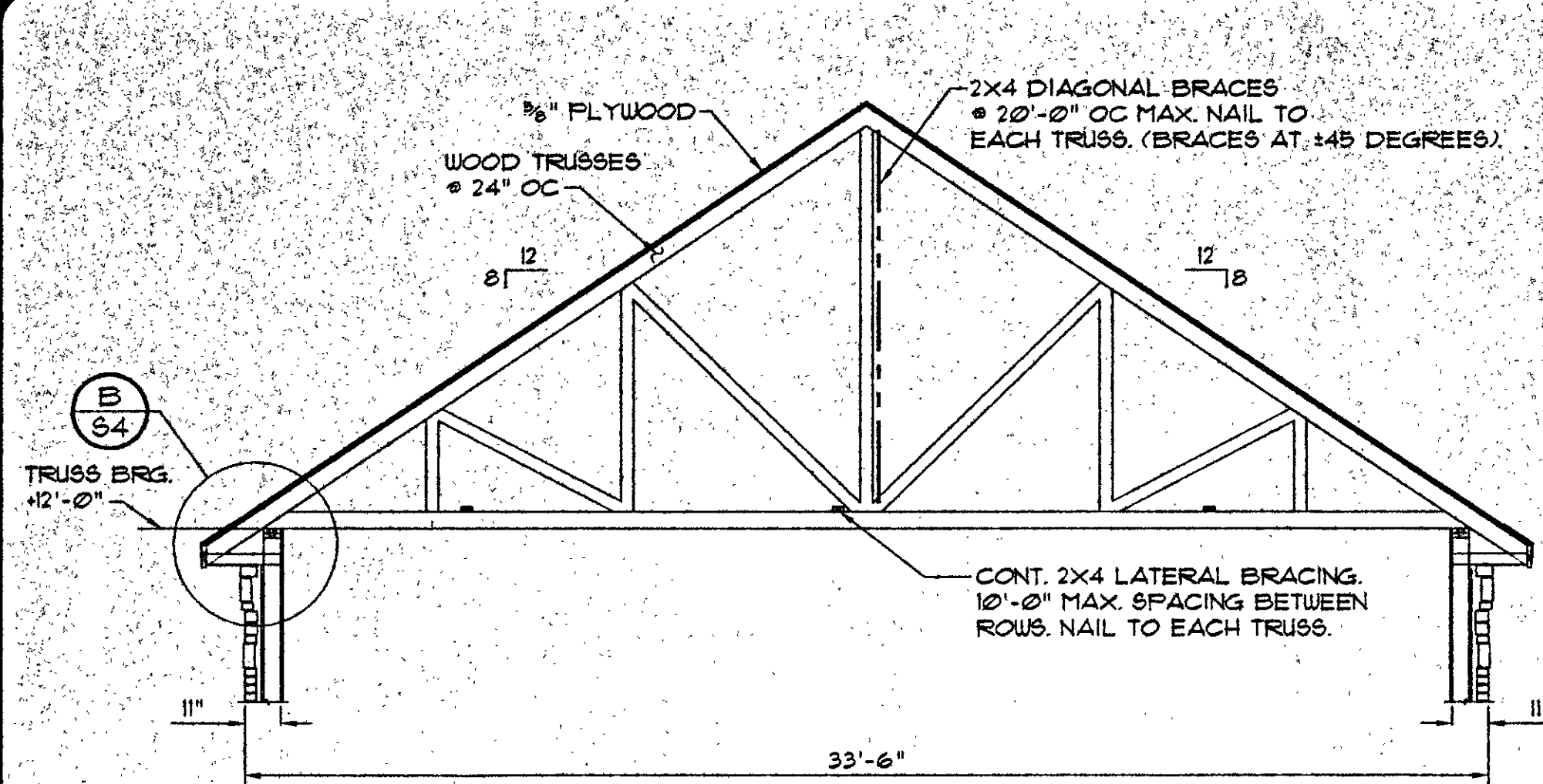


Lancaster Satellite DMH
CATAWBA M.H.C.
Lancaster, S.C.

STATE PROJECT NO. J12-9592-PC
Commission: C93-1510
Date: OCT. 23, 1995

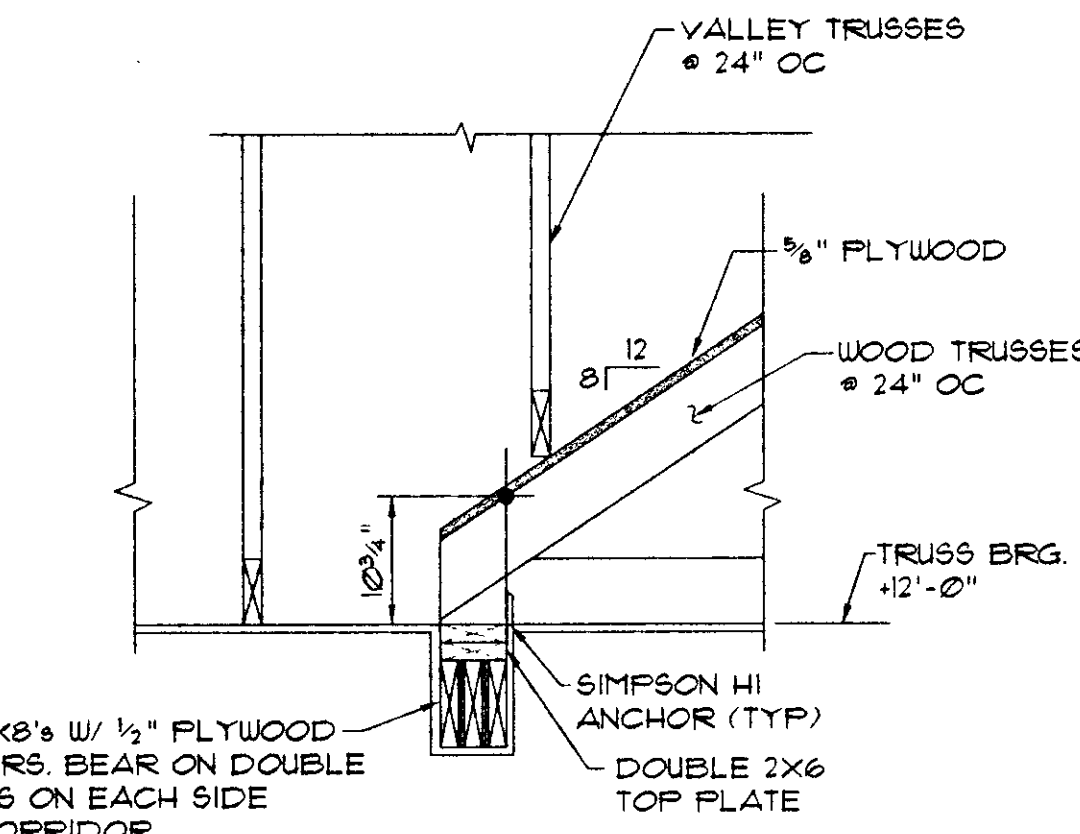
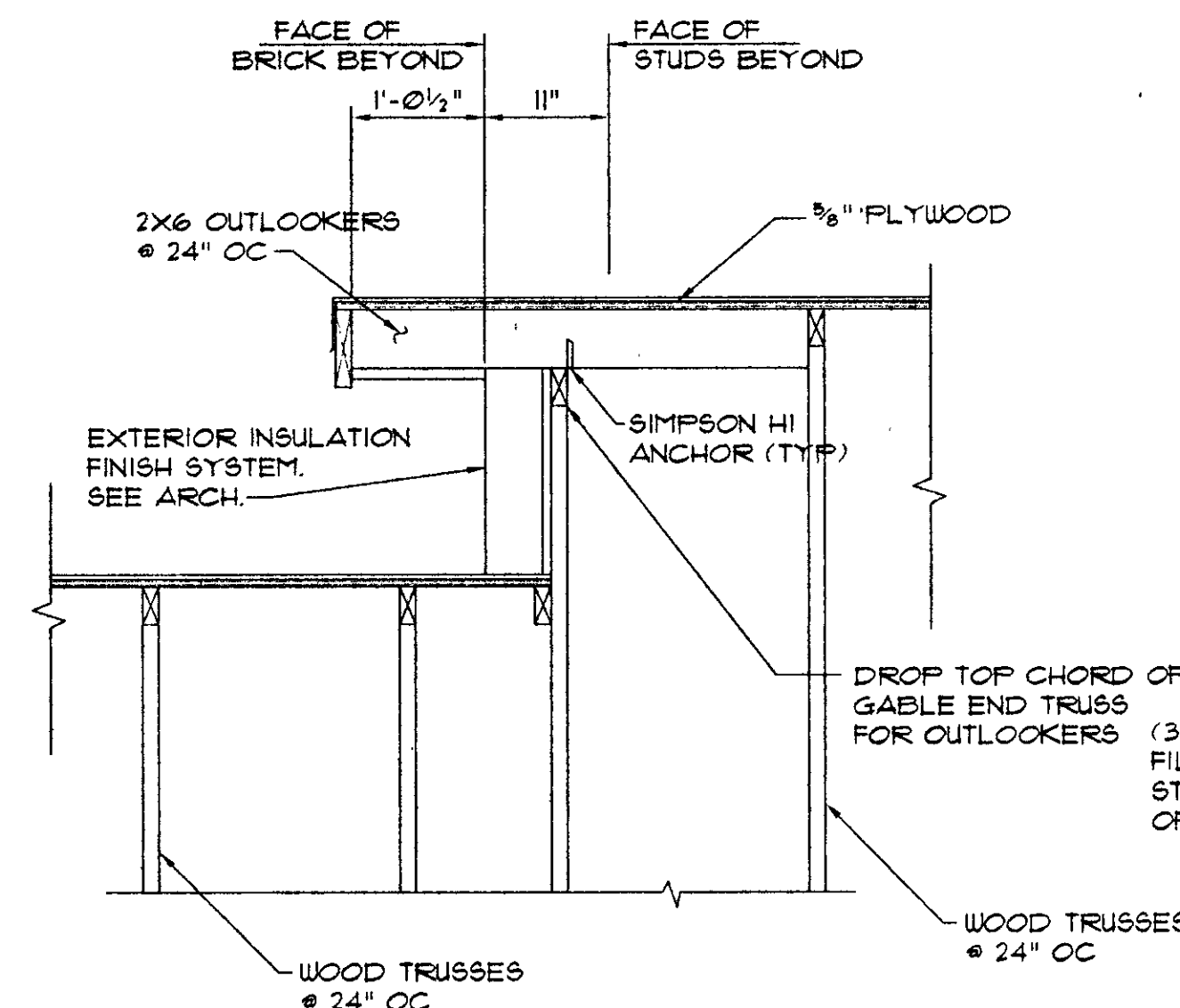
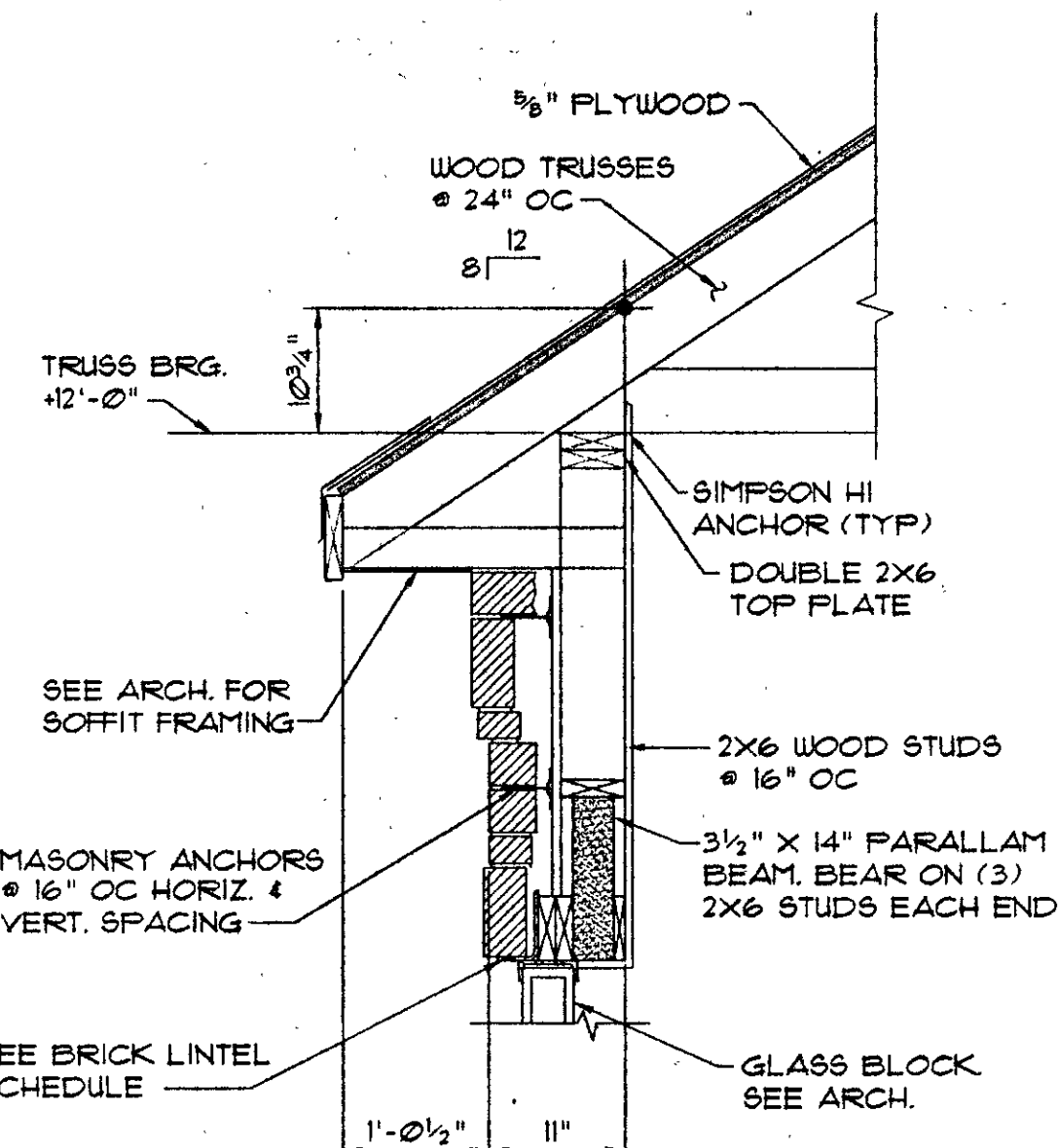
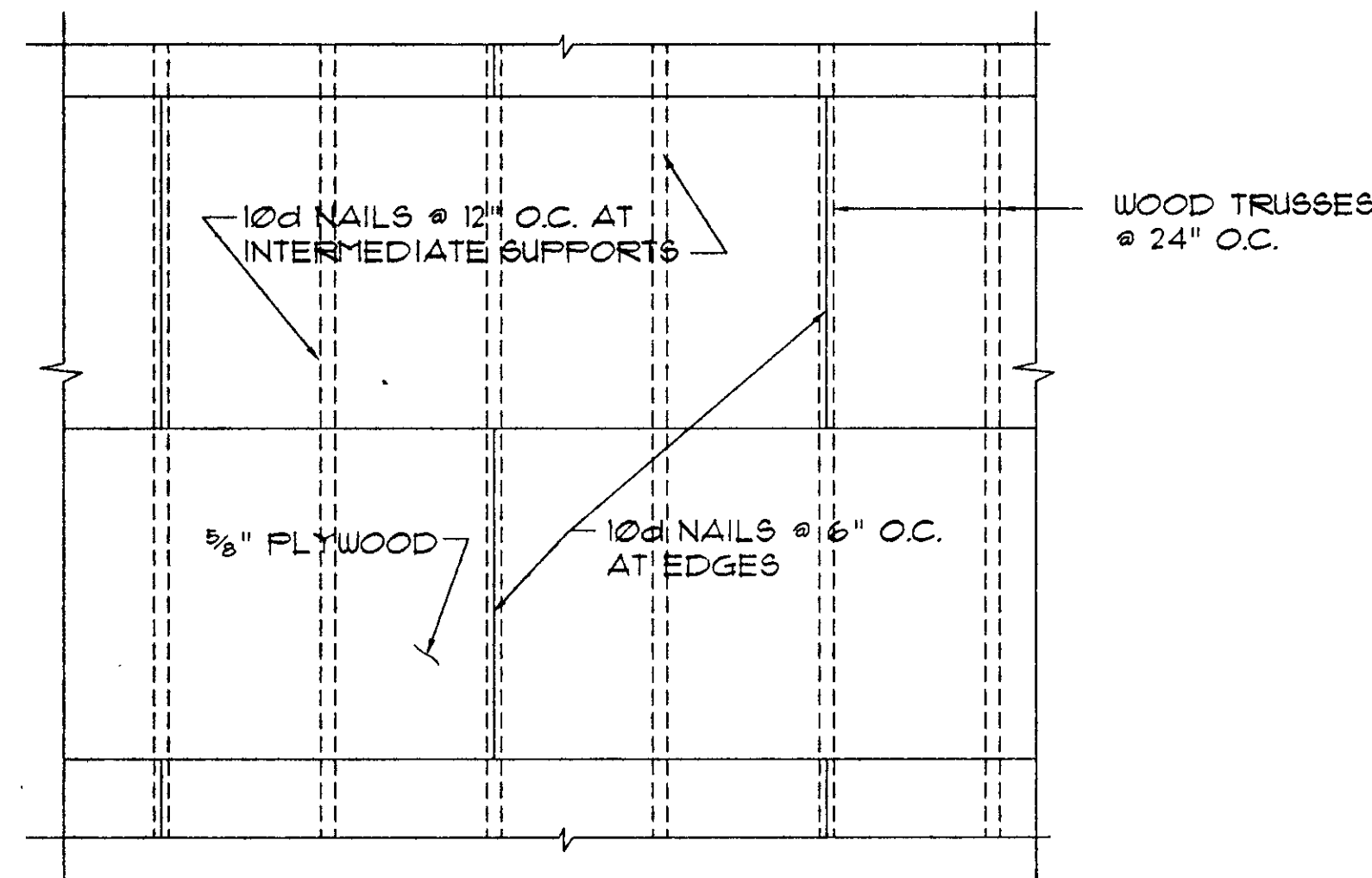
S-3

of 4 Sheets

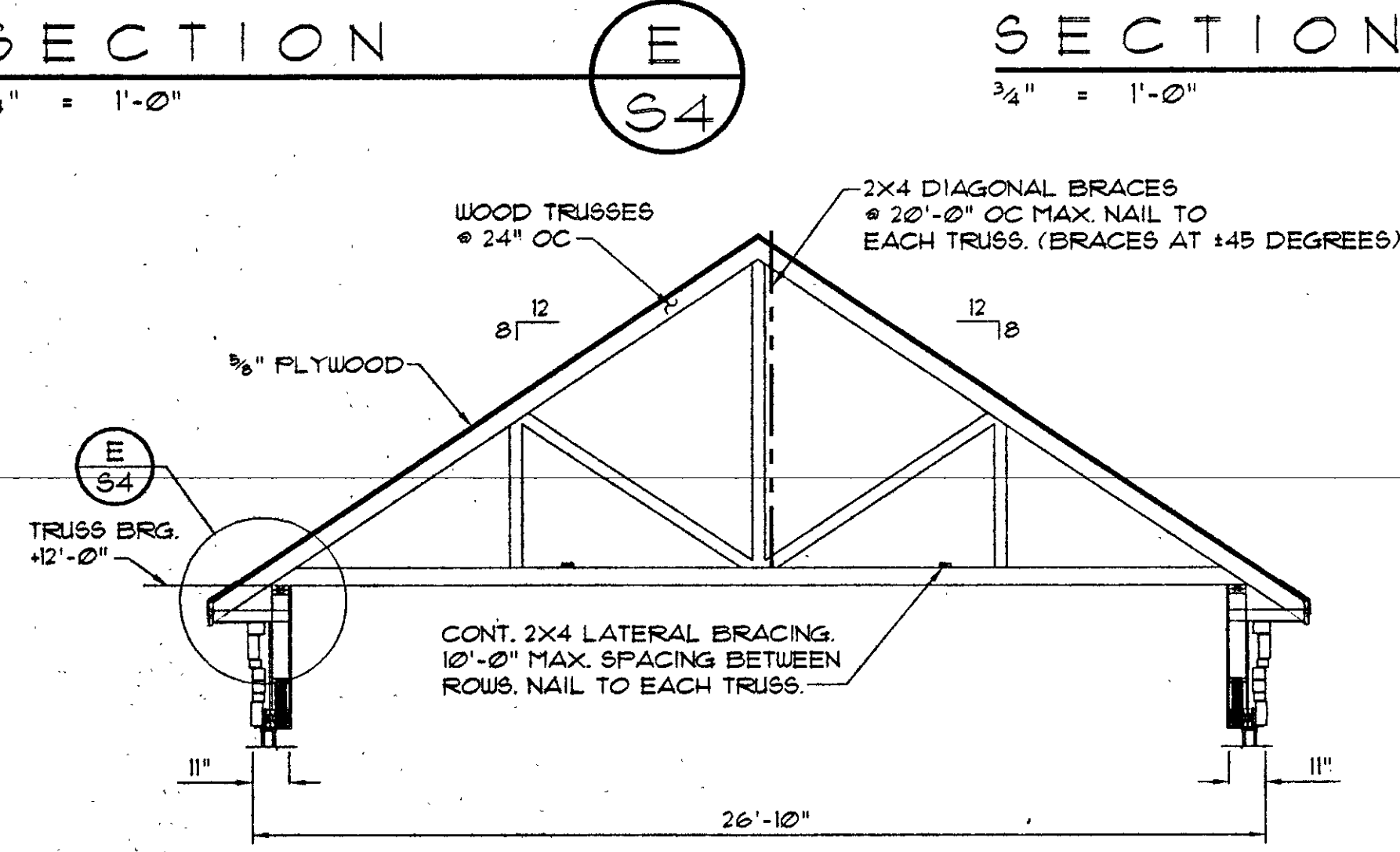
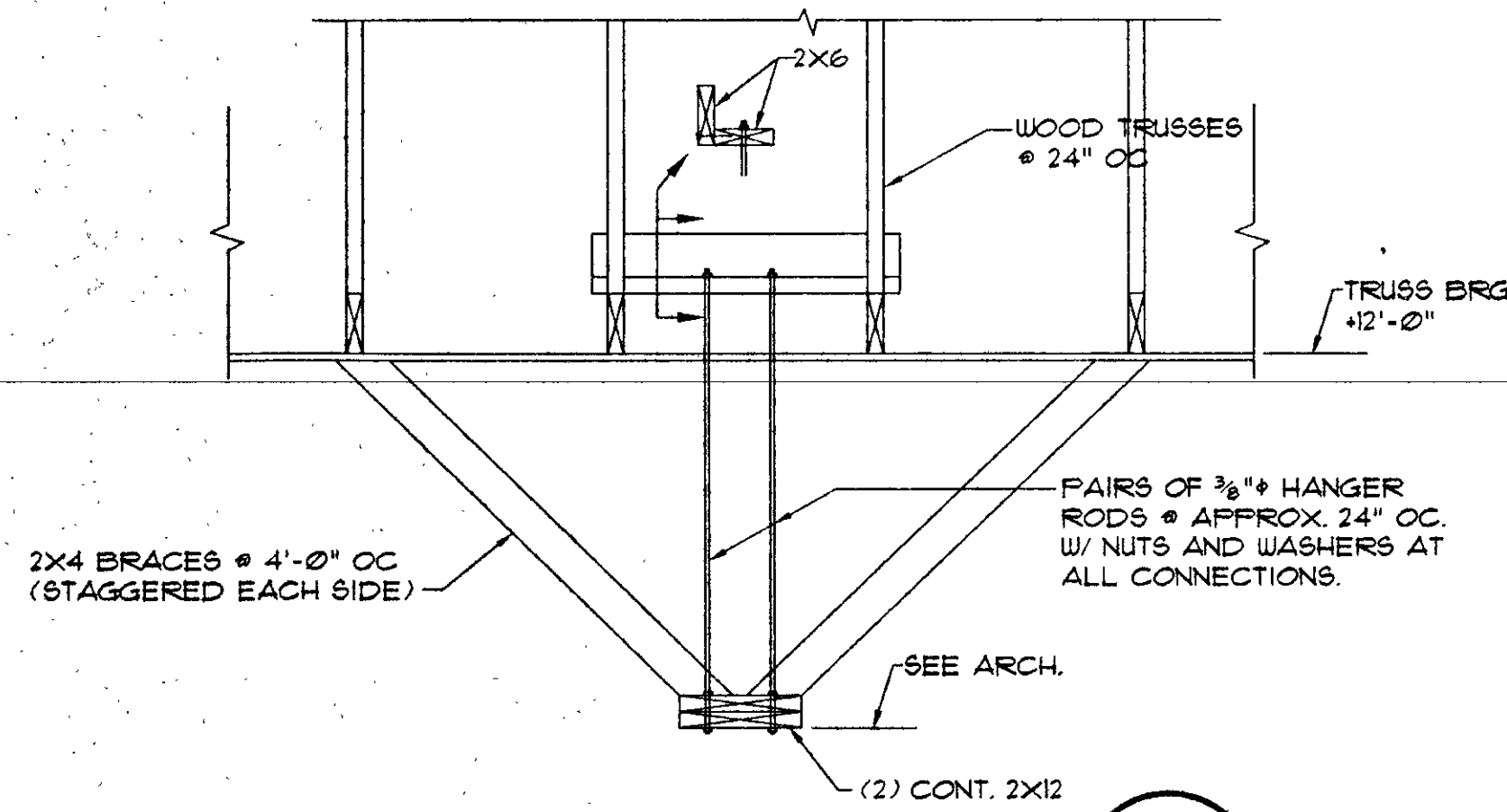


NOTES:
1. FLYWOOD SHEATHING FOR SHEARWALLS NOT SHOWN.
SEE DWG. S2 FOR LOCATIONS OF SHEARWALLS AND
SEE C/S3 FOR SHEARWALL DETAILS.

NOTES:
1. FLYWOOD SHEATHING FOR SHEARWALLS NOT SHOWN.
SEE DWG. S2 FOR LOCATIONS OF SHEARWALLS AND
SEE C/S3 FOR SHEARWALL DETAILS.

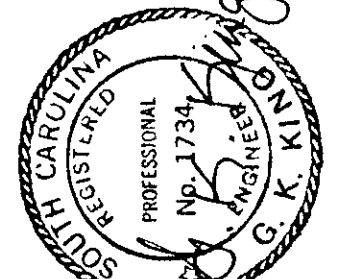


TYPICAL PLYWOOD ROOF DECK NAILING DETAIL



Design Collaborative

ARCHITECTS PLANNERS / COLUMBIA SOUTH CAROLINA



DMH

Lancaster Satellite
CATAWBA M.H.C.
Lancaster, S.C.

STATE PROJECT NO. J12-9592-PC
Commission: C93-1510
Date: OCT. 23, 1995

S-4
of 4 Sheets